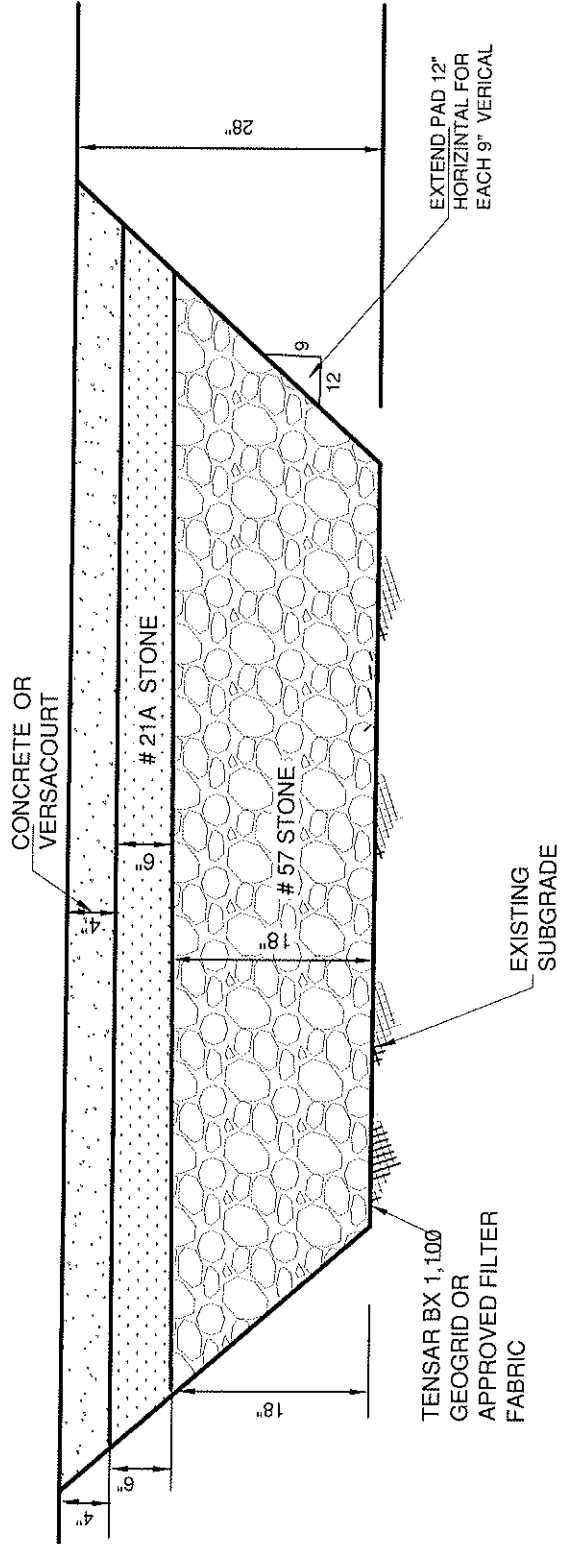


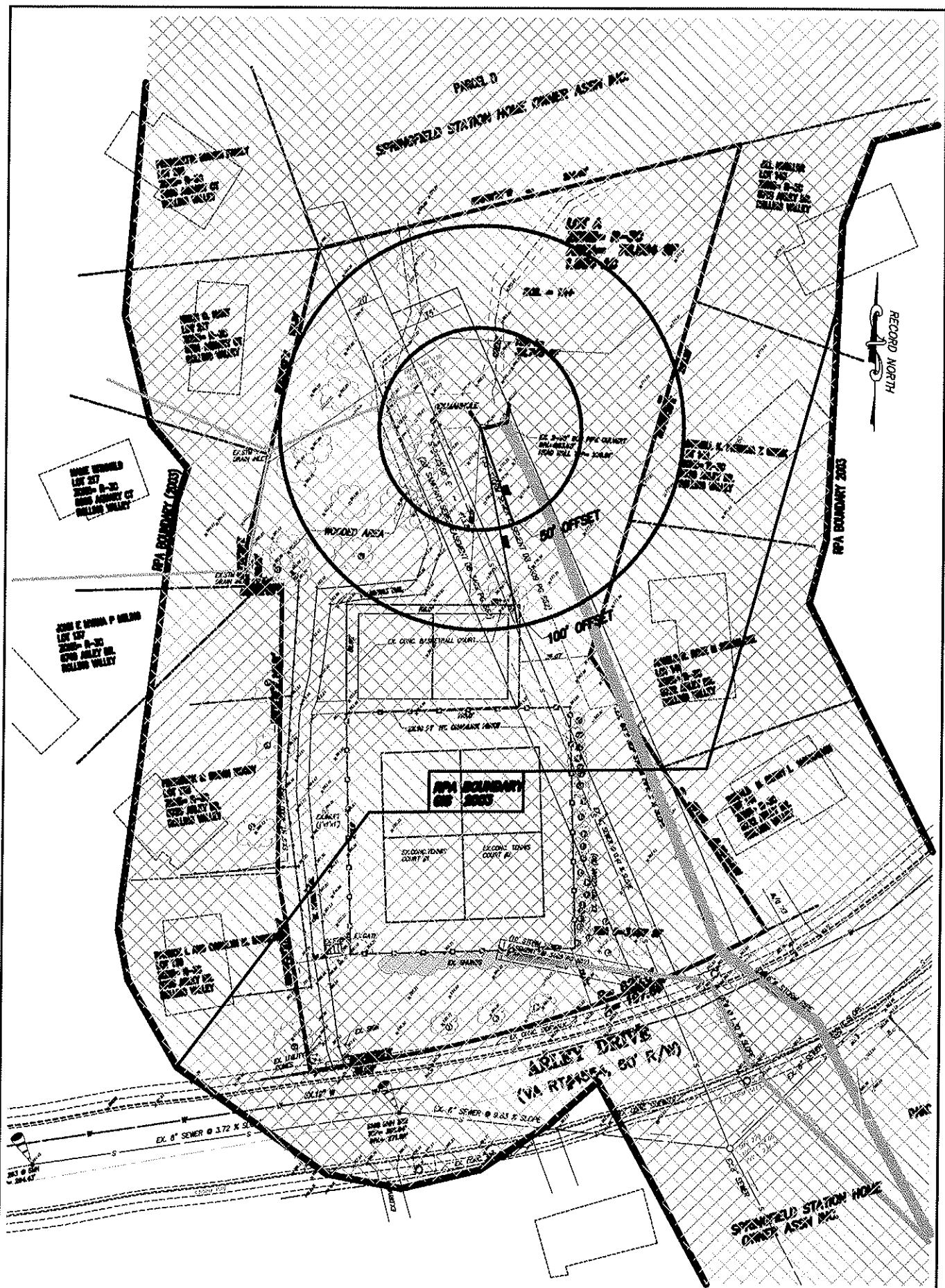


PROPOSED SPORT COURT'S PAD
 IMPROVEMENT
 SPRINGFIELD STATION HOME OWNERS'
 ASSOCIATION SPORTS COURT
 SPRINGFIELD, VIRGINIA 20152

GeoEnv *GeoEnv Engineers*
 Civil, Environmental & Geotechnical Engineering
 10875 Main Street, Suite 213
 Fairfax, VA 22030
 Tel. 703.591.7170
 Fax. 703.591.7074



PROPOSED SPORT COURT'S PAD IMPROVEMENT
 OPTIONS I & II




TAX MAP:	SP-3 (REV)-4
DATE:	NOV 2010
PREP. BY:	RAM
CHECKED BY:	AME
PROJECT #:	1000
SCALE:	1" = 20'
PAGE 3 OF 7	3

RPA BOUNDARY MAP (2003)
 SPRINGFIELD STATION HOME OWNERS' ASSOCIATION INC., ARLEY DRIVE, SPRINGFIELD PARCEL A, SEC 8B, ROLLING VALLEY
 SPRINGFIELD DISTRICT
 FARIFAX COUNTY, VIRGINIA 22153



REVISION BLOCK			
NO.	DESCRIPTION	REV. BY	APPROVED BY DATE
COUNTY PLAN NUMBER:			

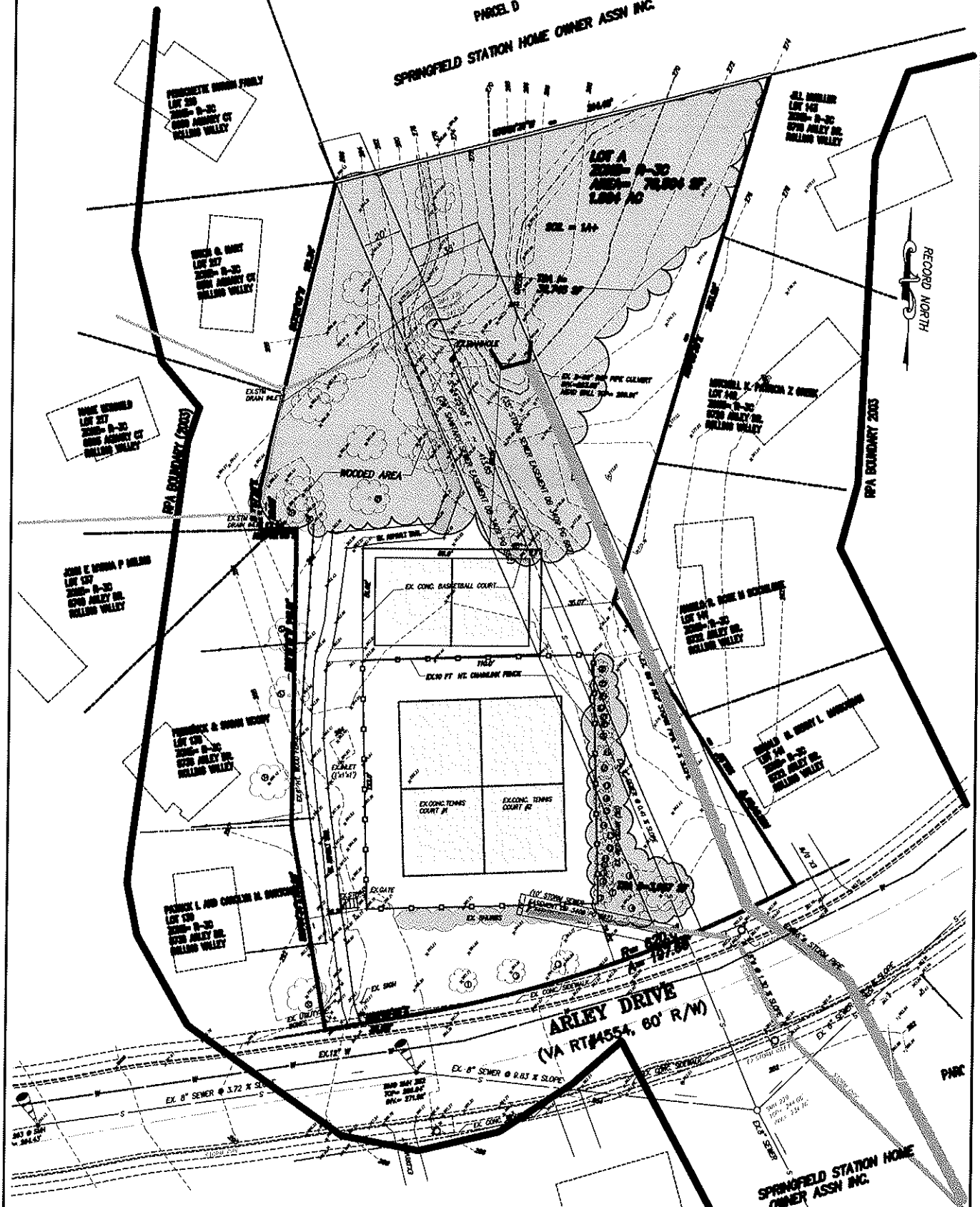

Engineers & Consultants, LLC
 Civil, Environmental & Geotechnical Engineering
 10020 Little Street, Suite 202
 Fairfax, VA 22030
 Tel: 703.681.7770
 Fax: 703.681.7099
 Web Site: www.geoenv.com

CONCEPT COVERED AREA :

TREE SAVE AREA = 32,740 SF
 TREE SAVE AREA = 3,137 SF
 TOTAL AREA = 35,877 SF
 % OF TREE COVERED AREA = 35,877/79,504 x 100 = 45.1% X

**EXISTING VEGETATION MAP:
 TREE COVERED AREAS/SPOTS OF GR. VEGET.**

PARCEL D
 SPRINGFIELD STATION HOME OWNER ASSN INC.



THE SITE CONTAINS SMALL, MEDIUM AND LARGE DECIDUOUS AND EVERGREEN TREES. THE SITE CONTAINS MAINLY MAPLE, OAK, PINE, HOLLY, CEDAR GREEN ASH. THE EXISTING VEGETATION SHALL BE PRESERVED TO THE MAXIMUM EXTENT POSSIBLE.

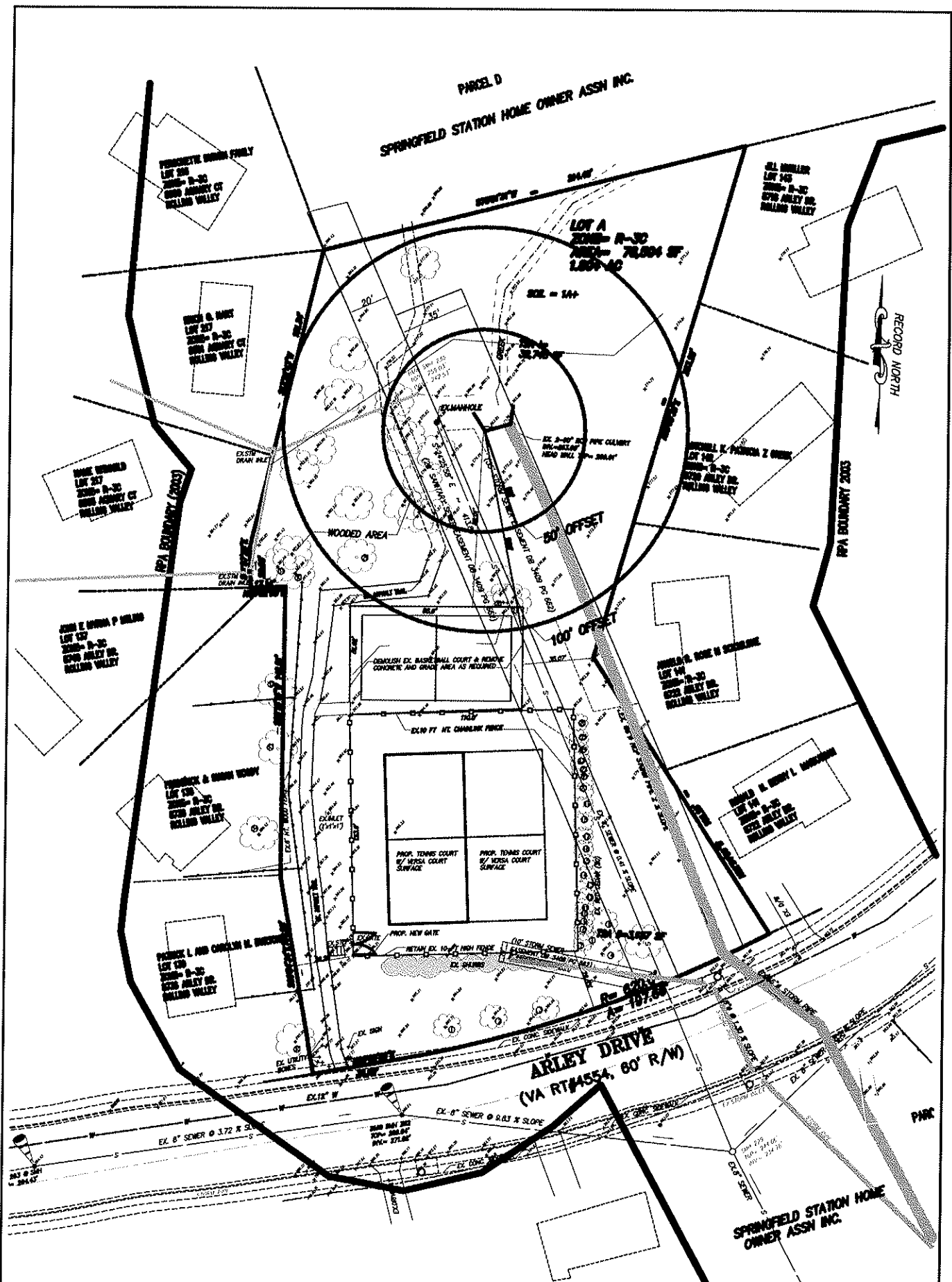
TAX MAP:	MP-3-(2008)-A
DATE:	NOV 2010
PREP. BY:	NAM
CHECKED BY:	AME
PROJECT #:	1080
SCALE:	1" = 20'
PAGE 5 OF 7	5

EXISTING VEGETATION PLAN
 SPRINGFIELD STATION HOME OWNERS' ASSOCIATION INC., ARLEY DRIVE, SPRINGFIELD PARCEL A, SEC 8B, ROLLING VALLEY
 SPRINGFIELD DISTRICT
 FARIFAX COUNTY, VIRGINIA 22153



REVISION BLOCK	
NO.	DESCRIPTION REV. BY APPROVED BY DATE
COUNTY PLAN NUMBER:	

GREEN ENGINEERS & CONSULTANTS, LLC
 Civil, Environmental & Geotechnical Engineering
 10520 Little Street, Suite 203
 Fairfax, VA 22030
 Tel: 703.691.7770
 Fax: 703.691.7090



PARCEL D
 SPRINGFIELD STATION HOME OWNER ASSN INC.

LOT A
 2000'-0" W-30"
 AREA= 78,804 SF
 1,800'-0"

SCALE = 1/4" = 1'-0"



RPA BOUNDARY 2003

ARLEY DRIVE
 (VA RT#1554, 60' R/W)

SPRINGFIELD STATION HOME
 OWNER ASSN INC.

TITLE SHEET	SP-3 (REV)-A
DATE	NOV 2010
PREP. BY	RAM
CHECKED BY	AKC
PROJECT	1500
SCALE	1" = 20'
PAGE 6(1) OF 7	6 (1)

PRELIMINARY PLAN (OPTION 1)
 SPRINGFIELD STATION HOME OWNERS'
 ASSOCIATION INC., ARLEY DRIVE, SPRINGFIELD
 PARCEL A, SEC 8B, ROLLING VALLEY
 SPRINGFIELD DISTRICT
 FARIFAX COUNTY, VIRGINIA 22153



NO.	DESCRIPTION	REV.	BY	APPROVED BY	DATE

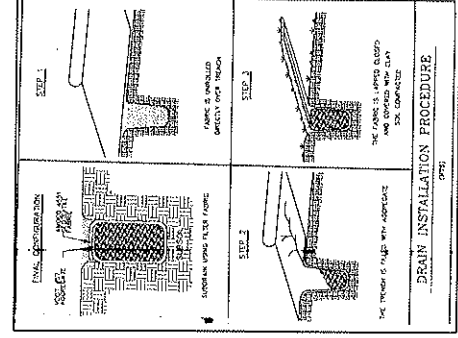
COUNTY PLAN NUMBER:

GeoEnv Engineers & Consultants, LLC
 Civil, Environmental & Geotechnical Engineering
 10000 Little Street, Suite 203
 Fairfax, VA 22030
 Tel: 703.896.7770
 Fax: 703.896.7094
 Web Site: geoenv.com



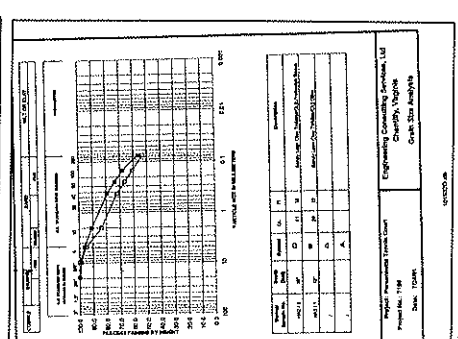
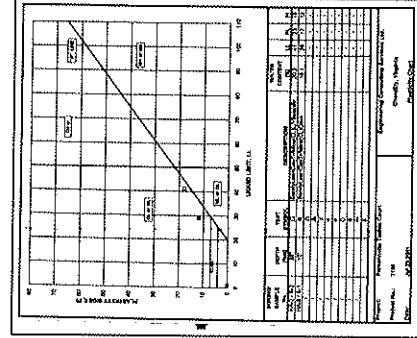
COUNTY PLAN NUMBER: _____
 DEPARTMENT: _____
 REVIEW BY APPROVED BY DATE: _____
 APPROVED BY: _____
 DATE: _____

Engineers & Consultants, LLC
 1700 Westpark Drive, Suite 100
 Fairfax, VA 22031
 Tel: 703.261.1100
 Fax: 703.261.1101
 www.ecsllc.com



Soil Profile Data Table

Profile No.	Depth (ft)	Soil Description	Moisture (%)	Specific Gravity	Unit Weight (pcf)	Void Ratio	Porosity (%)	Compression Index (Cc)	Liquid Limit (LL)	Plastic Limit (PL)	Shrinkage (%)	Swelling Potential (SP)	Soil Classification
1	0-1	Topsoil	15	2.65	110	0.55	45	0.05	20	10	1	Low	US-1
2	1-2	Light Clay	25	2.70	115	0.60	48	0.10	25	15	2	Low	US-2
3	2-3	Medium Clay	35	2.75	120	0.65	50	0.15	30	20	3	Medium	US-3
4	3-4	Heavy Clay	45	2.80	125	0.70	52	0.20	35	25	4	High	US-4

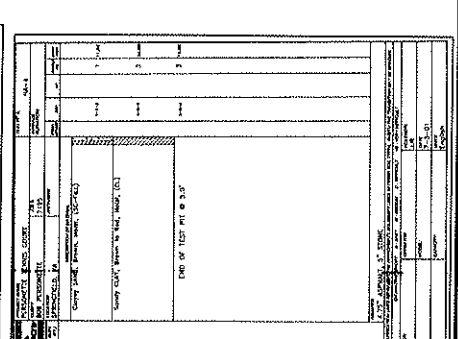


Soil Profile Data Table (continued)

Profile No.	Depth (ft)	Soil Description	Moisture (%)	Specific Gravity	Unit Weight (pcf)	Void Ratio	Porosity (%)	Compression Index (Cc)	Liquid Limit (LL)	Plastic Limit (PL)	Shrinkage (%)	Swelling Potential (SP)	Soil Classification
5	4-5	Very Heavy Clay	55	2.85	130	0.75	55	0.25	40	30	5	Very High	US-5
6	5-6	Clay with Silt	40	2.70	115	0.60	48	0.10	25	15	2	Low	US-6
7	6-7	Silt with Clay	30	2.65	110	0.55	45	0.05	20	10	1	Low	US-7
8	7-8	Sand with Silt	20	2.60	105	0.50	42	0.02	15	5	0	None	US-8

Soil Profile Data Table (continued)

Profile No.	Depth (ft)	Soil Description	Moisture (%)	Specific Gravity	Unit Weight (pcf)	Void Ratio	Porosity (%)	Compression Index (Cc)	Liquid Limit (LL)	Plastic Limit (PL)	Shrinkage (%)	Swelling Potential (SP)	Soil Classification
9	8-9	Clay with Silt	25	2.70	115	0.60	48	0.10	25	15	2	Low	US-9
10	9-10	Silt with Clay	15	2.65	110	0.55	45	0.05	20	10	1	Low	US-10
11	10-11	Sand with Silt	10	2.60	105	0.50	42	0.02	15	5	0	None	US-11
12	11-12	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-12

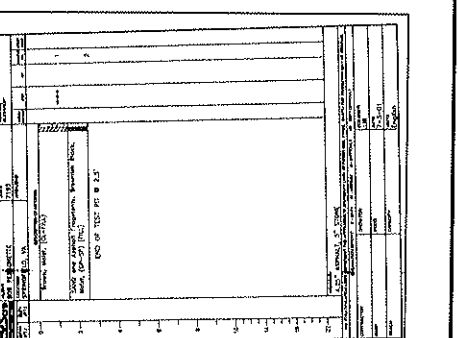


Soil Profile Data Table (continued)

Profile No.	Depth (ft)	Soil Description	Moisture (%)	Specific Gravity	Unit Weight (pcf)	Void Ratio	Porosity (%)	Compression Index (Cc)	Liquid Limit (LL)	Plastic Limit (PL)	Shrinkage (%)	Swelling Potential (SP)	Soil Classification
13	12-13	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-13
14	13-14	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-14
15	14-15	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-15
16	15-16	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-16

Soil Profile Data Table (continued)

Profile No.	Depth (ft)	Soil Description	Moisture (%)	Specific Gravity	Unit Weight (pcf)	Void Ratio	Porosity (%)	Compression Index (Cc)	Liquid Limit (LL)	Plastic Limit (PL)	Shrinkage (%)	Swelling Potential (SP)	Soil Classification
17	16-17	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-17
18	17-18	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-18
19	18-19	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-19
20	19-20	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-20



Soil Profile Data Table (continued)

Profile No.	Depth (ft)	Soil Description	Moisture (%)	Specific Gravity	Unit Weight (pcf)	Void Ratio	Porosity (%)	Compression Index (Cc)	Liquid Limit (LL)	Plastic Limit (PL)	Shrinkage (%)	Swelling Potential (SP)	Soil Classification
21	20-21	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-21
22	21-22	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-22
23	22-23	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-23
24	23-24	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-24

Soil Profile Data Table (continued)

Profile No.	Depth (ft)	Soil Description	Moisture (%)	Specific Gravity	Unit Weight (pcf)	Void Ratio	Porosity (%)	Compression Index (Cc)	Liquid Limit (LL)	Plastic Limit (PL)	Shrinkage (%)	Swelling Potential (SP)	Soil Classification
25	24-25	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-25
26	25-26	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-26
27	26-27	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-27
28	27-28	Sand	5	2.60	105	0.50	42	0.02	15	5	0	None	US-28

CONCLUSIONS:
 The soil profiles described herein were obtained from the site of the proposed development. The soil profiles are classified as follows: US-1 through US-28. The soil profiles are generally composed of sand, silt, and clay. The soil profiles are generally well-sorted and have a low plasticity. The soil profiles are generally composed of sand, silt, and clay. The soil profiles are generally well-sorted and have a low plasticity. The soil profiles are generally composed of sand, silt, and clay. The soil profiles are generally well-sorted and have a low plasticity.

RECOMMENDATIONS:
 The soil profiles described herein were obtained from the site of the proposed development. The soil profiles are classified as follows: US-1 through US-28. The soil profiles are generally composed of sand, silt, and clay. The soil profiles are generally well-sorted and have a low plasticity. The soil profiles are generally composed of sand, silt, and clay. The soil profiles are generally well-sorted and have a low plasticity. The soil profiles are generally composed of sand, silt, and clay. The soil profiles are generally well-sorted and have a low plasticity.

NOTES:
 1. The soil profiles described herein were obtained from the site of the proposed development. The soil profiles are classified as follows: US-1 through US-28. The soil profiles are generally composed of sand, silt, and clay. The soil profiles are generally well-sorted and have a low plasticity. The soil profiles are generally composed of sand, silt, and clay. The soil profiles are generally well-sorted and have a low plasticity. The soil profiles are generally composed of sand, silt, and clay. The soil profiles are generally well-sorted and have a low plasticity.

ENGINEERS & CONSULTANTS, LLC
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VERSACOURT™



The Ultimate Athletic Court System!



VERSACOURT

VersaCourt game courts are magnets for people of all ages providing many hours of healthy, enjoyable, interactive exercise. VersaCourt's custom designed indoor and outdoor courts incorporate the most recent advancements in engineering and manufacturing technology.

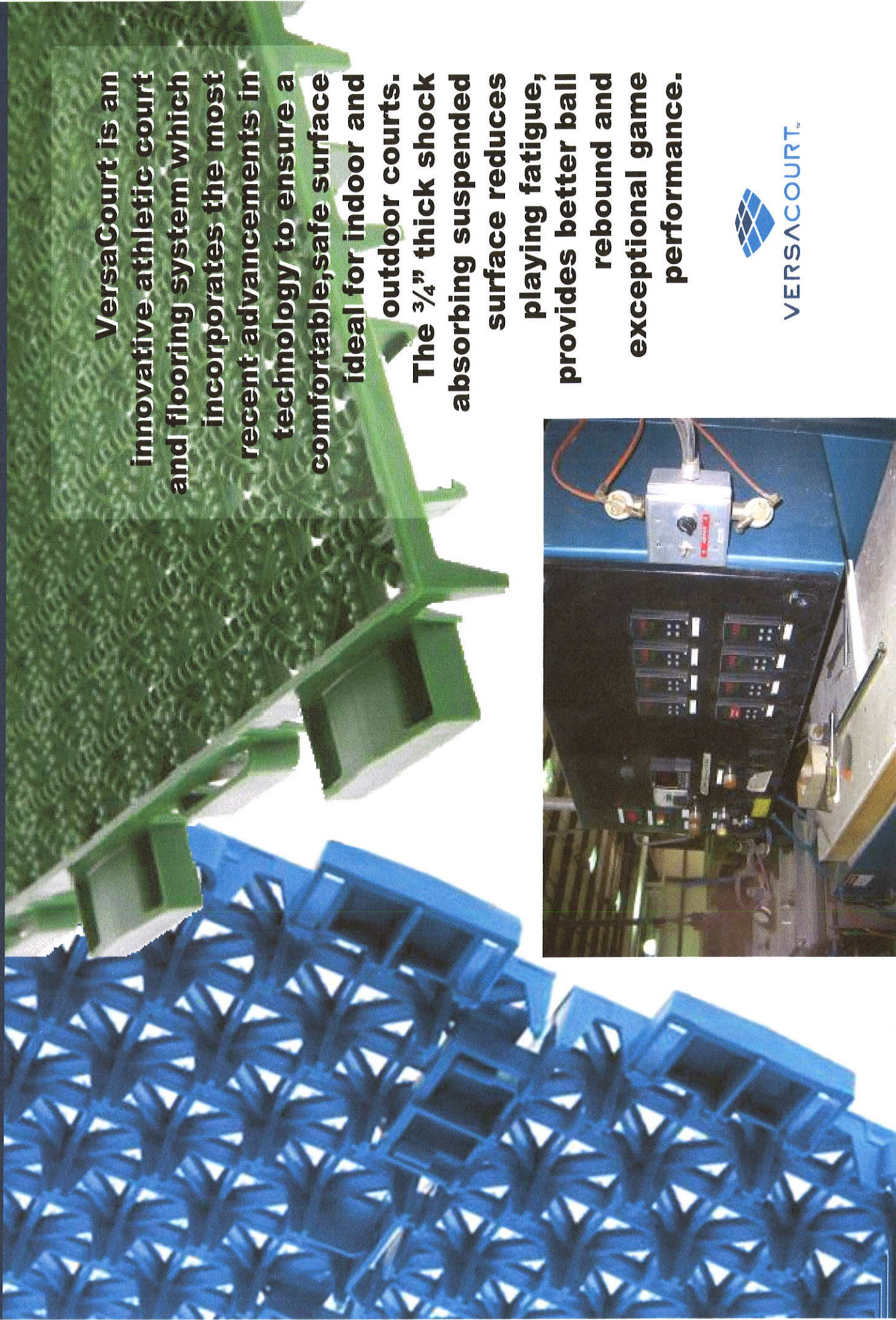
It's our drive for perfection that makes VersaCourt the best performing surface on the market today. Ideal for home and commercial applications, the sheer beauty and high-performance of our courts make it the right choice for all settings.

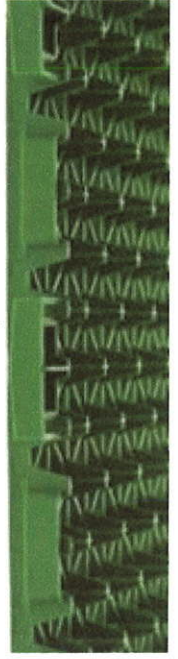
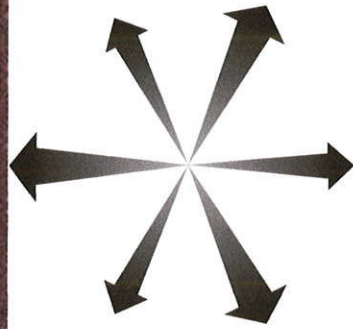
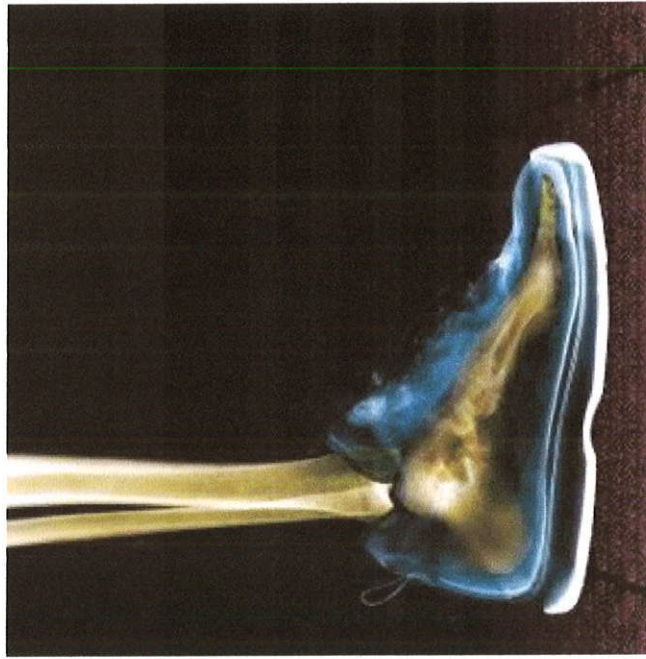


Cutting Edge Technology

VersaCourt is an innovative athletic court and flooring system which incorporates the most recent advancements in technology to ensure a comfortable, safe surface ideal for indoor and outdoor courts.

The $\frac{3}{4}$ " thick shock absorbing suspended surface reduces playing fatigue, provides better ball rebound and exceptional game performance.





Awesome Physical Benefits

VersaCourt's unsurpassed physical benefits stem from its cutting edge tiles, which are 3/4" thick and feature multi-directional flex technology. The expansion joints & spring tabbing mechanism between the tiles allow slight side-to-side movement. VersaCourt's design also allows for a slight vertical give in the tile. The multi-directional flex features reduce fatigue and stress on joints, ligaments and lower back. This built-in flex action makes VersaCourt one of the safest and healthiest court surfaces on the market today. Orthopedic doctors have even installed VersaCourt for their own use just because of the reduction in stress on joints and ligaments and the prevention of sports injuries.



The ultimate multi-use game court



A Virtual Family Magnet



Because of its sheer beauty,
high performance,
cool and comfortable playing surface,
the courts are magnets for people of
all ages, providing many hours of
healthy enjoyable exercise.





ENGINEERING CONSULTING SERVICES, LTD.
Geotechnical • Construction Materials • Environmental

July 26, 2001

Mr. Bob Personette
8800 Aquary Court
Springfield, Virginia 22153

ECS Job No. 7195

Reference: Report of Subsurface Exploration Services for Personette Tennis Courts,
Springfield, Fairfax County, Virginia

Dear Mr. Personette:

As authorized by your acceptance of our proposal No. 12588-GPR, Engineering Consulting Services, Ltd. has completed the subsurface exploration for the above referenced project. The subsurface exploration consisted of performing six hand augers at various locations around the two tennis courts in Springfield, Virginia. In conjunction with the hand augers Dynamic Cone Penetrometer Tests (DCP) were performed at various depths to determine the relative density of the in-place soils. This exploration was performed to determine the possible causes of the cracks of the existing pavement along the tennis courts. The approximate location of the exploration points are shown on the Boring Location Diagram provided in the Appendix.

Background

The above referenced site consists of two tennis courts as well as a basketball court off of Arley Drive in Springfield, Virginia. Upon initial observation of the tennis courts, cracking was evident throughout the pavement of the tennis court. Based on the existing site topography, fill was most likely placed for constructing the tennis courts. Based on the information provided to us, it is understood that the tennis court has received several overlays in recent years. However, the cracking of the tennis court continues to occur. In order to better evaluate the subsurface conditions, a series of six hand augers were performed.

Exploration Procedure

A hand auger probe consists of a four inch diameter hole drilled with a portable auger bucket. Each of the hand auger probes were extended to bucket refusal within the existing soil matrix. The soil samples recovered were visually classified in the field on the basis of texture and plasticity in accordance with the Unified Soil Classification System (USCS). The group symbols for each soil type are indicated in parenthesis following the soil description on the hand auger logs. A brief description of the USCS is included with this letter. The stratification lines designating the interfaces between earth materials on the hand auger logs and profiles are

14026 Thunderbolt Place, Suite 100, Chantilly, Virginia 20151 • (703) 471-8400 • FAX (703) 834-5527

Aberdeen, MD • Atlanta, GA • Austin, TX • Baltimore, MD • Chantilly, VA • Charlotte, NC • Chicago, IL • Cornelia, GA* • Dallas, TX • Danville, VA* • Frederick, MD • Fredericksburg, VA
Greensboro, NC • Greenville, SC • Norfolk, VA • Research Triangle Park, NC • Richmond, VA • Roanoke, VA • Williamsburg, VA • Wilmington, NC • Winchester, VA*

*Testing Services Only

approximate; in-situ, the transitions may be gradual. Hand auger results are include with this letter.

Additionally, Dynamic Cone Penetrometer (DCP) tests were performed in accordance with ASTM STP-399 at selected depths in each of the hand auger probes to evaluate the relative strength of the anticipated foundation bearing soils. The DCP tests consist of dropping a 15 pound weight over a distance of 20 inches. The number of blows required to advance the cone 1.75 inches, determines the DCP blow count. The blow counts can be empirically correlated to the Standard Penetration Test (SPT) and subsequently the bearing capacity and relative density can be evaluated for specific soil types. The results of the DCP testing are also included in the text of this report.

Laboratory Testing Program

Representative soil samples were selected and tested in our laboratory to check field classifications and to determine pertinent engineering properties. The laboratory testing program included visual classifications, moisture content tests, and Atterberg Limits. All data obtained from the laboratory tests are attached with this letter report.

An experienced soil engineer classified each soil sample on the basis of texture and plasticity in accordance with the Unified Soil Classification System. The group symbols for each soil type are indicated in parentheses following the soil descriptions on the tables in the following section.

The soil samples will be retained in our laboratory for a period of 60 days, after which, they will be discarded unless other instructions are received as to their disposition.

Results of Subsurface Exploration

The exploration points encountered approximately 4 to 5 inches of asphalt pavement followed by 3 to 5 inches of stone. Beneath the pavement and subbase material, existing fill soils were encountered to depths of 2 to 5 feet below the existing ground surface. The fill soils consisted of CLAY (CL, CH), and SAND and Asphalt fragments (GP, SP). Natural soils were encountered in only two of the hand augers and consisted of primarily Sandy CLAY and CLAY (CL, CH). In addition to the soils found in our hand augers, perched water was noted in the vicinity of Hand Auger No. 6 at a depth of approximately 3 feet. A graphical log of each of the hand augers is in the attached Appendix.

Engineering Recommendations

The presence of medium to highly plastic clay under the majority of the existing tennis court, indicates that the cracking is most likely a result of the high shrink-swell potential of the soils.

In addition, the presence of perched water underneath the tennis court is also a contributing factor.

Due to the presence of medium to highly plastic soils and the possibility of perched groundwater, we are recommending the following of two options. The first involves complete removal of the existing asphalt and gravel subbase followed by a 2-foot undercut of the existing soils and replacement with properly placed low plasticity, preferably granular engineered fill. The second involves only a limited undercut utilizing lime stabilization in the upper 1 foot of soils. Regardless of the alternative chosen we recommend the installation of a french drain system around the existing tennis court. Each of the two options are discussed in detail in subsequent paragraphs.

The first and most conservative of these options is to completely remove the existing asphalt and stone subbase. Following the removal, we recommend that 2 feet of existing soil, most notably the high plasticity clay, be undercut and removed. Following the removal we recommend the area be thoroughly proofrolled to identify any soft or unsuitable material. Proofrolling involves using a loaded dump truck, having an axle weight of at least 10 tons, over the exposed subgrade to identify any localized soft or unsuitable material which should be removed. Once a stable subgrade is achieved, fill should be placed in order to achieve the existing finished tennis court elevation. We recommend that fill used to support any of the proposed tennis court should be placed in lifts not exceeding 8 inches in loose thickness, moisture conditioned to within 2% of the optimum moisture content, and compacted to at least 95% of the dry density obtained from ASTM D-698, Standard Proctor Method. However, we recommend that the upper 1 foot be compacted to at least 100% of the aforementioned maximum dry density.

It is critical that any soil placed as fill should be of an approved material, free of organic matter or debris, be a non-frost susceptible soil, and have a liquid limit and plasticity index less than 40 and 15, respectively. The surficial soils encountered in our hand auger do not appear suitable for reuse as engineered fill. We recommend that SM or more granular soils be utilized for backfill and establishing the proposed subgrade.

As an alternative to a complete 2 foot undercut of the existing clay soils, we recommend the use of lime stabilization. Lime stabilization would involve the removal of the asphalt and stone subbase and applying hydrated lime to the upper 1 foot of the exposed subgrade. We recommend treating the subgrade soils to a depth of 1 foot with an estimated 4% to 6% hydrated lime. Following lime stabilization fill placement can proceed as detailed in the previous paragraph.

Regardless of either scenario chosen we recommend that a french drain be installed around the tennis court that will serve to drain water away from the tennis courts. A french detail is attached with this letter.

Pavement Considerations

Once the subgrade has been properly remediated or stabilized, replacement of the existing tennis court can begin. With the appropriate routine maintenance a new pavement section should have a serviceable lifespan of 20 years. We recommend that a minimum 2 inches of surface course asphalt in conjunction with a 4 inch subbase aggregate (21A) be used to restore the pavement section. In addition, geogrid reinforcement may be applicable underneath the subgrade aggregate. A geogrid reinforcing layer would serve to reduce the potential for differential settlement in areas of the court where some localized soft material may still exist. Tensar BX1100 geogrid is typically used in this application. If used, the geogrid should be placed in accordance with the manufacturer's recommendations. It should be noted that a final pavement design should be performed based on the CBR value of the final subgrade soils.

Closing

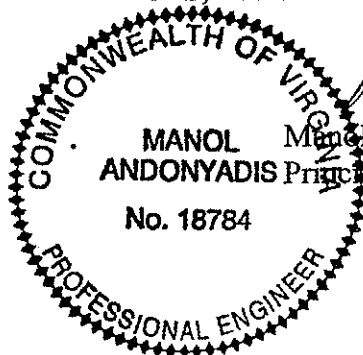
This report has been prepared for your use and for the use of other design professionals in design and construction planning for the proposed improvements to the existing tennis court in Springfield, Virginia. Any changes in the design or additional information regarding the history of the existing tennis court, should be brought to our attention in order to determine any affect on the recommendations presented in this report. We also recommend that we be given the opportunity to assist you in reviewing the prospective bids so that we can determine whether or not the contractors have properly interpreted the project requirements. Additionally, we recommend that you contact us prior to the construction phase, so that we can coordinate the field observations and testing during the execution of the proposed improvements.

We have enjoyed being of service to you. If you have any questions with regard to the information and recommendations presented in this report, or if we can be of further assistance, please do not hesitate to contact us.

Respectfully,

ENGINEERING CONSULTING SERVICES, LTD.


Chad W. Yeatts, E.I.T.
Assistant Project Engineer





MANOL P. Andonyadis, P.E.
ANDONYADIS Principal Engineer

No. 18784

APPENDIX

Unified Soil Classification System

Hand Auger Logs

Laboratory Test Results

French Drain Detail

Boring Location Diagram/Vicinity Map



PROJECT NAME: PERSONETTE TENNIS COURT TEST PIT #: HA-1

CLIENT: BOB PERSONETTE

JOB #: 7195

SURFACE ELEVATION:

DEPTH (FT.) ELEV. (FT.)

LOCATION: SPRINGFIELD, VA

ARCH/ENG:

EXCAV. EFFORT DCP QP SAMPLE NO. MOIST. CONT.

0
2
4
6
8
10
14
18
22

DESCRIPTION OF MATERIAL
Brown, Moist, (CL-FILL)
SAND and Asphalt Fragments, Brownish Black, Moist, (GP-SP) [FILL]
END OF TEST PIT @ 2.5'

10-9-9
1
2

REMARKS:
4.25" ASPHALT, 5" STONE

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES, IN-SITU THE TRANSITION MAY BE GRADUAL

EXCAVATION EFFORT: E - EASY M - MEDIUM D - DIFFICULT VD - VERY DIFFICULT

CONTRACTOR: OPERATOR: ECS ENGR: JJR

MAKE: MODEL: DATE: 7-3-01

REACH: CAPACITY: UNITS: English



PROJECT NAME:
PERSONETTE TENNIS COURT

TEST PIT #:
HA-2

CLIENT:
BOB PERSONETTE

JOB #:
7195

SURFACE ELEVATION:

DEPTH (FT.)

ELEV. (FT.)

LOCATION:
SPRINGFIELD, VA

ARCH./ENG:

EXCAV. EFFORT

0
2
4
6
8
10
14
18
22

DESCRIPTION OF MATERIAL

Sandy CLAY, Trace Roots, Gray, Moist, (CL-FILL)

Sandy CLAY, Reddish Brown, Moist, (CH-FILL)

Sandy CLAY, Grayish Brown, Moist, (CL-CH)

END OF TEST PIT @ 5.0'

DCP	QP	SAMPLE NO.	MOIST. CONT.
5-6-4		1	10.9%
5-4-4		2	20.2%
8-9-7		3	23.4%

REMARKS:
5" ASPHALT, 3" STONE

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES, IN-SITU THE TRANSITION MAY BE GRADUAL

EXCAVATION EFFORT: E - EASY M - MEDIUM D - DIFFICULT VD - VERY DIFFICULT

CONTRACTOR:

OPERATOR:

ECS ENGR:
JJR

MAKE:

MODEL:

DATE:
7-3-01

REACH:

CAPACITY:

UNITS:
English



PROJECT NAME:
PERSONETTE TENNIS COURT

TEST PIT #:
HA-3

CLIENT:
BOB PERSONETTE

JOB #:
7195

SURFACE
ELEVATION:

LOCATION:
SPRINGFIELD, VA

ARCH./ENG:

EXCAV. EFFORT	DCP	QP	SAMPLE NO.	MOIST. CONT.
---------------	-----	----	------------	--------------

DEPTH (FT.)

0

2

4

6

8

10

14

18

22

ELEV. (FT.)

DESCRIPTION OF MATERIAL

CLAY, Grayish Brown, Moist, (CH-FILL)

Sandy CLAY, Brown, Moist, (CL-FILL)

CLAY, Trace Gravel, Reddish Brown, Moist, (CH-FILL)

END OF TEST PIT @ 5.0'

	4-5-5		1	18.1%
	4-3-4		2	18.5%
	5-7-5		3	22.7%

REMARKS:
4.75" ASPHALT, 4.5" STONE

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES, IN-SITU THE TRANSITION MAY BE GRADUAL

EXCAVATION EFFORT: E - EASY M - MEDIUM D - DIFFICULT VD - VERY DIFFICULT

CONTRACTOR:

OPERATOR:

ECS ENGR:
JJR

MAKE:

MODEL:

DATE:
7-3-01

REACH:

CAPACITY:

UNITS:
English



PROJECT NAME:
PERSONETTE TENNIS COURT

TEST PIT #:
HA-4

CLIENT:
BOB PERSONETTE

JOB #:
7195

SURFACE
ELEVATION:

DEPTH
(FT.)

ELEV.
(FT.)

LOCATION:
SPRINGFIELD, VA

ARCH/ENG:

EXCAV.
EFFORT

DCP

QP

SAMPLE
NO.

MOIST.
CONT.

DESCRIPTION OF MATERIAL

Clayey SAND, Brown, Moist, (SC-FILL)

Sandy CLAY, Brown to Red, Moist, (CL)

END OF TEST PIT @ 5.0'

7-7-5

1

11.6%

4-5-6

2

24.5%

3-3-4

3

19.0%

REMARKS:

4.75" ASPHALT, 5" STONE

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES, IN-SITU THE TRANSITION MAY BE GRADUAL

EXCAVATION EFFORT: E - EASY M - MEDIUM D - DIFFICULT VD - VERY DIFFICULT

CONTRACTOR:

OPERATOR:

ECS ENG'R:

JJR

TAKE:

MODEL:

DATE:

7-3-01

REACH:

CAPACITY:

UNITS:

English



PROJECT NAME:
PERSONETTE TENNIS COURT

TEST PIT #:
HA-5

CLIENT:
BOB PERSONETTE

JOB #:
7195

SURFACE ELEVATION:

DEPTH (FT.)

ELEV. (FT.)

LOCATION:
SPRINGFIELD, VA

ARCH/ENG:

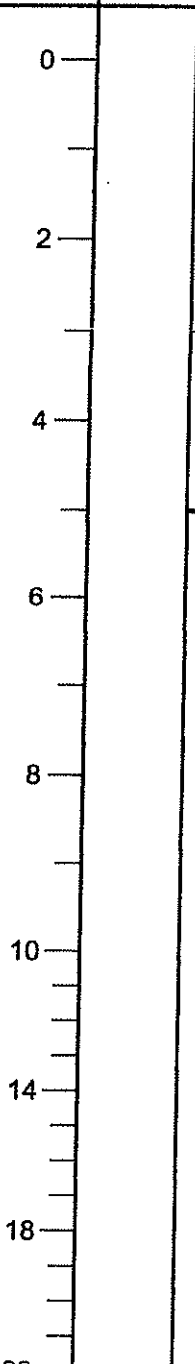
EXCAV. EFFORT

DCP

QP

SAMPLE NO.

MOIST. CONT.



DESCRIPTION OF MATERIAL

Sandy CLAY, Brown, Moist, (CL-CH) [FILL]



4-4-4

1

CLAY, Reddish Brown, Moist, (CH-Poss. FILL)



5-4-4

2

16.5%

END OF TEST PIT @ 5.0'

7-7-5

3

REMARKS:
4.25" ASPHALT, 4" STONE

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES, IN-SITU THE TRANSITION MAY BE GRADUAL

EXCAVATION EFFORT: E - EASY M - MEDIUM D - DIFFICULT VD - VERY DIFFICULT

CONTRACTOR:

OPERATOR:

ECS ENGR:
JJR

TAKE:

MODEL:

DATE:
7-3-01

REACH:

CAPACITY:

UNITS:
English



PROJECT NAME:
PERSONETTE TENNIS COURT

TEST PIT #:
HA-6

CLIENT:
BOB PERSONETTE

JOB #:
7195

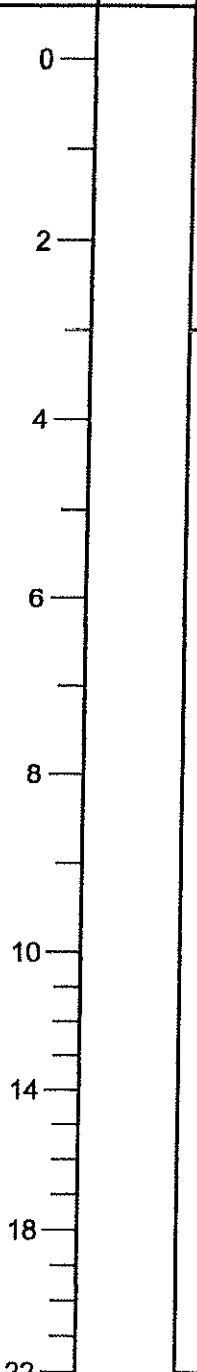
SURFACE ELEVATION:

DEPTH (FT.)
ELEV. (FT.)

LOCATION:
SPRINGFIELD, VA

ARCH/ENG:

EXCAV. EFFORT
DCP
QP
SAMPLE NO.
MOIST. CONT.



DESCRIPTION OF MATERIAL

CLAY, Grayish Brown, Moist, (CH-FILL)

BUCKET REFUSAL @ 3.0'

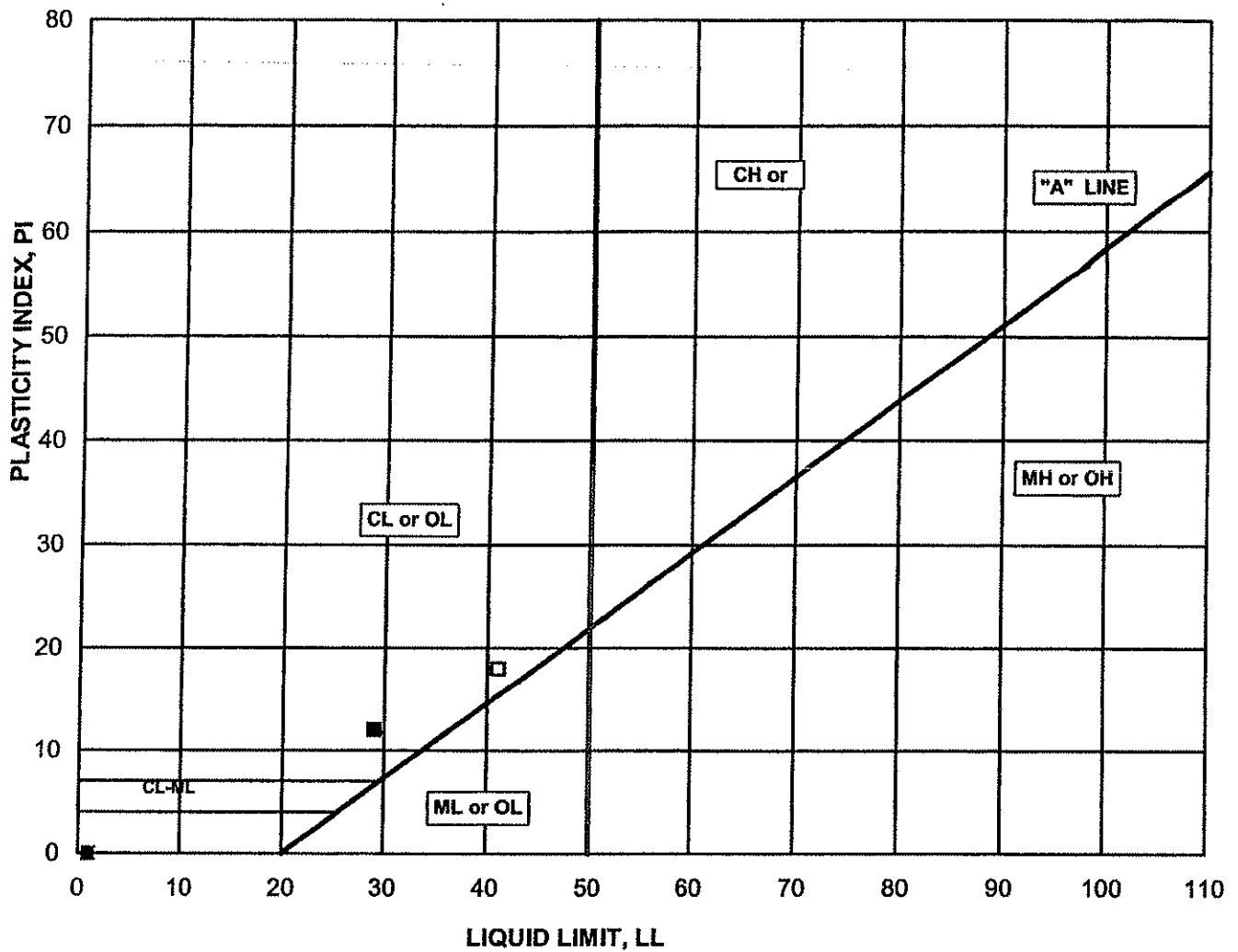
EXCAV. EFFORT	DCP	QP	SAMPLE NO.	MOIST. CONT.
	6-7-6		1	
	4-4-3		2	

REMARKS:
4" ASPHALT, 4" STONE WATER ENCOUNTERED AT 3'

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES, IN-SITU THE TRANSITION MAY BE GRADUAL

EXCAVATION EFFORT: E - EASY M - MEDIUM D - DIFFICULT VD - VERY DIFFICULT

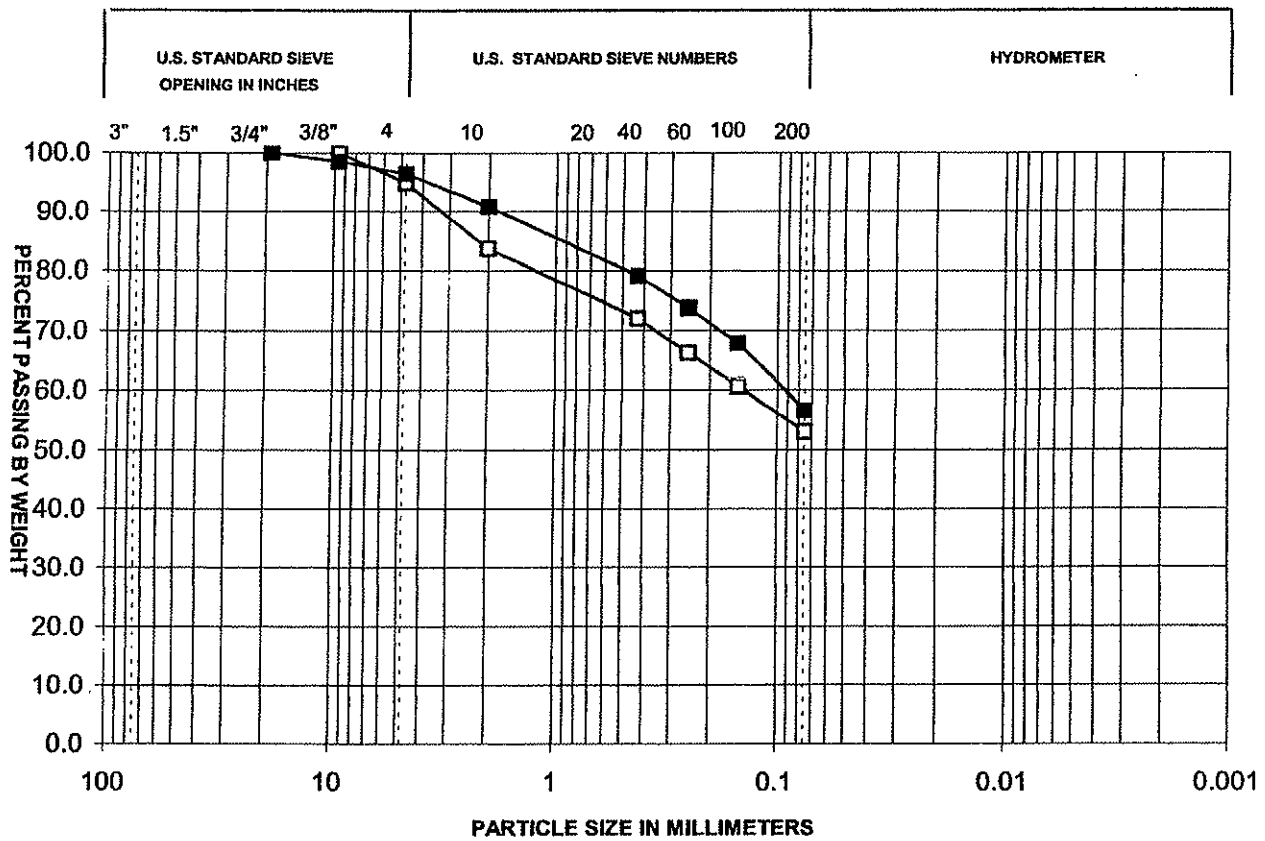
CONTRACTOR:	OPERATOR:	ECS ENGR: JJR
MAKE:	MODEL:	DATE: 7-3-01
REACH:	CAPACITY:	UNITS: English



BORING/ SAMPLE No.	DEPTH (feet)	TEST SYMBOL	DESCRIPTION	WATER CONTENT (%)	LL	PL	PI
HA-2 / S-2	35"	□	SandyLeanClayTr/Mica(CL)L/YllwishBr	20.2	41	23	18
HA-3 / S-1	12"	■	SandyLeanClayTr/Mica(CL)Olive	18.1	29	17	12
/		△			-	-	-
/		▲			-	-	-
/		X			-	-	-
/		○			-	-	-
/		●			-	-	-
/		◇			-	-	-
/		◆			-	-	-
/		+			-	-	-
/		X			-	-	-

Project:	Personnette Tennis Court	Engineering Consulting Services Ltd.
Project No.:	7195	Chantilly, Virginia
Date:	Jul 23, 2001	Plasticity Chart

COBBLE	GRAVEL		SAND			SILT OR CLAY
		FINE	COARSE		FINE	



Boring/ Sample No.	Depth (feet)	Symbol	LL	PI	Description
HA2 / 2	35"	□	41	18	Sandy Lean Clay Tr/Mica(CL)L/Yellowish Brown
HA3 / 1	12"	■	29	12	Sandy Lean Clay Tr/Mica(CL) Olive
/		△			
/		▲			

Project: Personnette Tennis Court
 Project No.: 7195
 Date: 7/24/01

Engineering Consulting Services, Ltd
 Chantilly, Virginia
 Grain Size Analysis

Unified Soil Classification System (ASTM D-2487)

Major Divisions		Group Symbols	Typical Names	Laboratory Classification Criteria		
Coarse-grained soils (More than half of material is larger than No. 200 sieve size)	Gravels (More than half of coarse fraction is larger than No. 4 sieve size)	Clean gravels (Little or no fines)	GW	Well-graded gravels, gravel-sand mixtures, little or no fines	$C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3 Not meeting all gradation requirements for GW Atterberg limits below "A" line or P.I. less than 4 Atterberg limits below "A" line with P.I. greater than 7 $C_u = \frac{D_{60}}{D_{10}}$ greater than 6; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3 Not meeting all gradation requirements for SW Atterberg limits above "A" line or P.I. less than 4 Atterberg limits above "A" line with P.I. greater than 7	
			GP	Poorly graded gravels, gravel-sand mixtures, little or no fines		
		Gravels with fines (Appreciable amount of fines)	GM ^a	d		Silty gravels, gravel-sand-silt mixtures
				u		
			GC	Clayey gravels, gravel-sand-clay mixtures		
	Sands (More than half of coarse fraction is smaller than No. 4 sieve size)	Clean sands (Little or no fines)	SW	Well-graded sands, gravelly sands, little or no fines		
			SP	Poorly graded sands, gravelly sands, little or no fines		
		Sands with fines (Appreciable amount of fines)	SM ^a	d	Silty sands, sand-silt mixtures	
				u		
			SC	Clayey sands, sand-clay mixtures		
Fine-grained soils (More than half material is smaller than No. 200 sieve)	Silts and clays (Liquid limit less than 50)	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity			
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays			
		OL	Organic silts and organic silty clays of low plasticity			
	Silts and clays (Liquid limit greater than 50)	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts			
		CH	Inorganic clays of high plasticity, fat clays			
		OH	Organic clays of medium to high plasticity, organic silts			
	Pt	Peat and other highly organic soils				

Determine percentages of sand and gravel from grain-size curve. Depending on percentage of fines (fraction smaller than No. 200 sieve size), coarse-grained soils are classified as follows:
 Less than 5 per cent
 More than 12 per cent
 5 to 12 per cent

GW, GP, SW, SP
 GM, GC, SM, SC
 Borderline cases requiring dual symbols^b

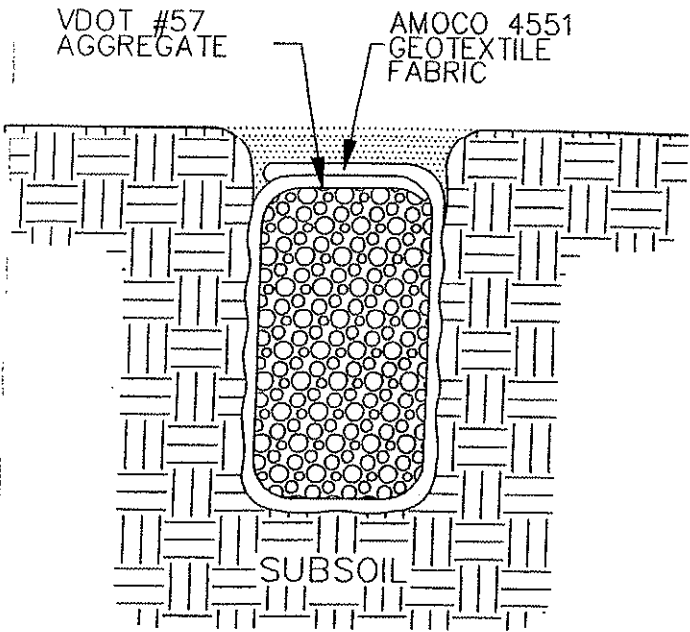
Plasticity Chart

The Plasticity Chart plots Plasticity Index (y-axis, 0 to 60) against Liquid Limit (x-axis, 0 to 100). A diagonal line labeled 'A' line separates the upper regions (CH, OH and MH) from the lower regions (CL, ML and OL). A hatched zone is located between the CL-ML boundary and the ML and OL region.

^aDivision of GM and SM groups into subdivisions of d and u are for roads and airfields only. Subdivision is based on Atterberg limits; suffix d used when L.L. is 28 or less and the P.I. is 6 or less; the suffix u used when L.L. is greater than 28.

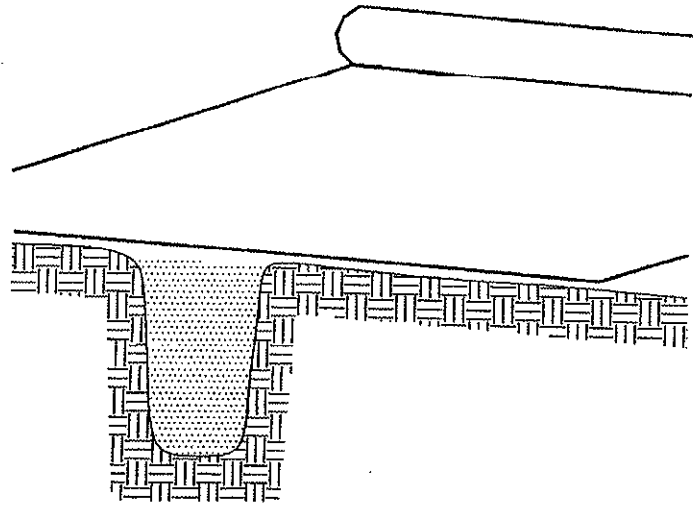
^bBorderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC, well-graded gravel-sand mixture with clay binder.

FINAL CONFIGURATION



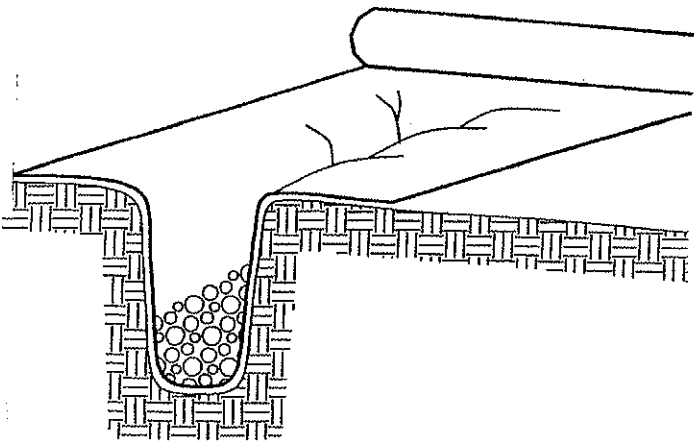
SUBDRAIN USING FILTER FABRIC

STEP 1



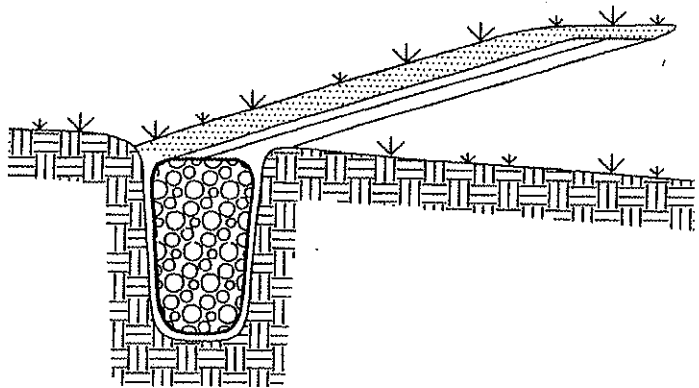
FABRIC IS UNROLLED
DIRECTLY OVER TRENCH

STEP 2



THE TRENCH IS FILLED WITH AGGREGATE

STEP 3



THE FABRIC IS LAPPED CLOSED
AND COVERED WITH CLAY
SOIL COMPACTED

DRAIN INSTALLATION PROCEDURE

(NTS)